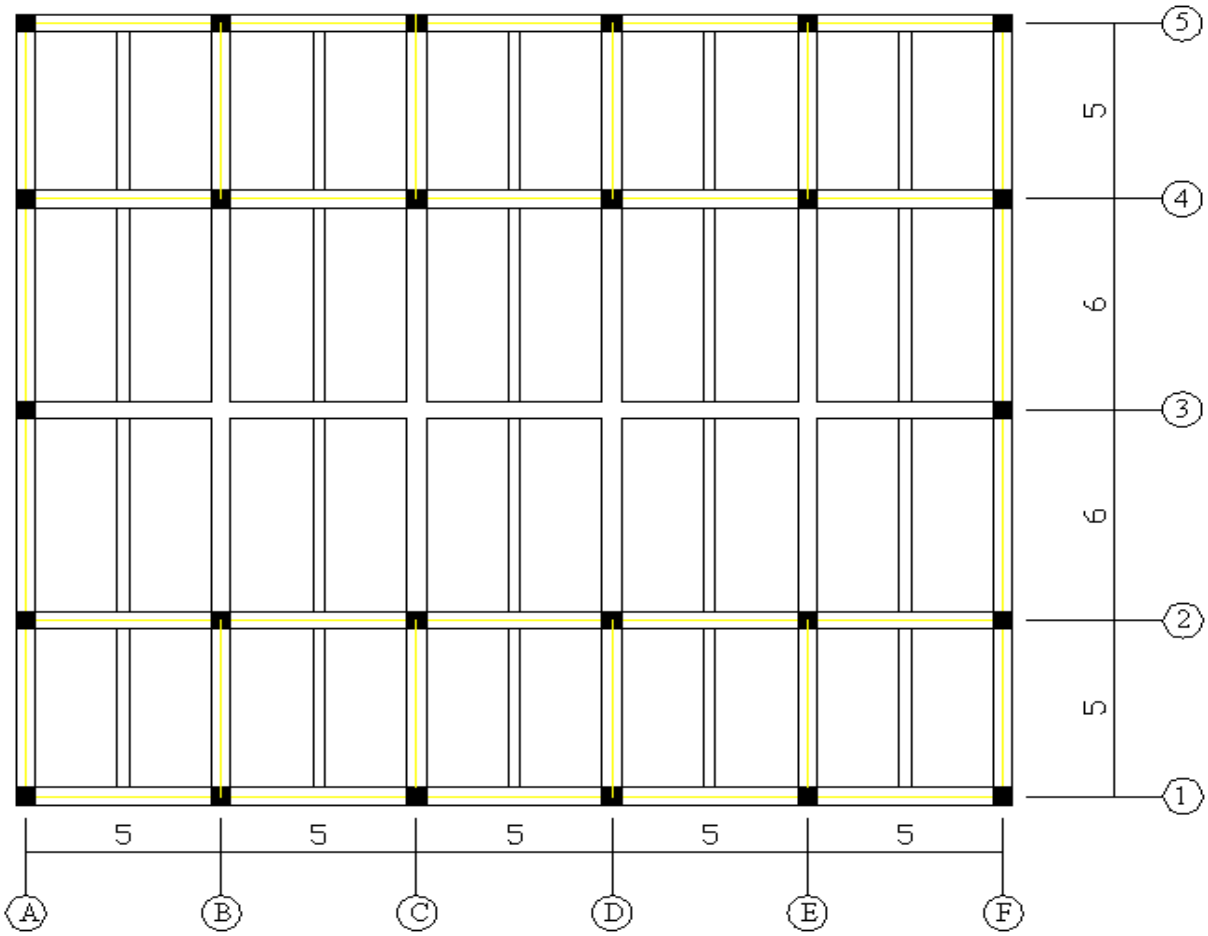
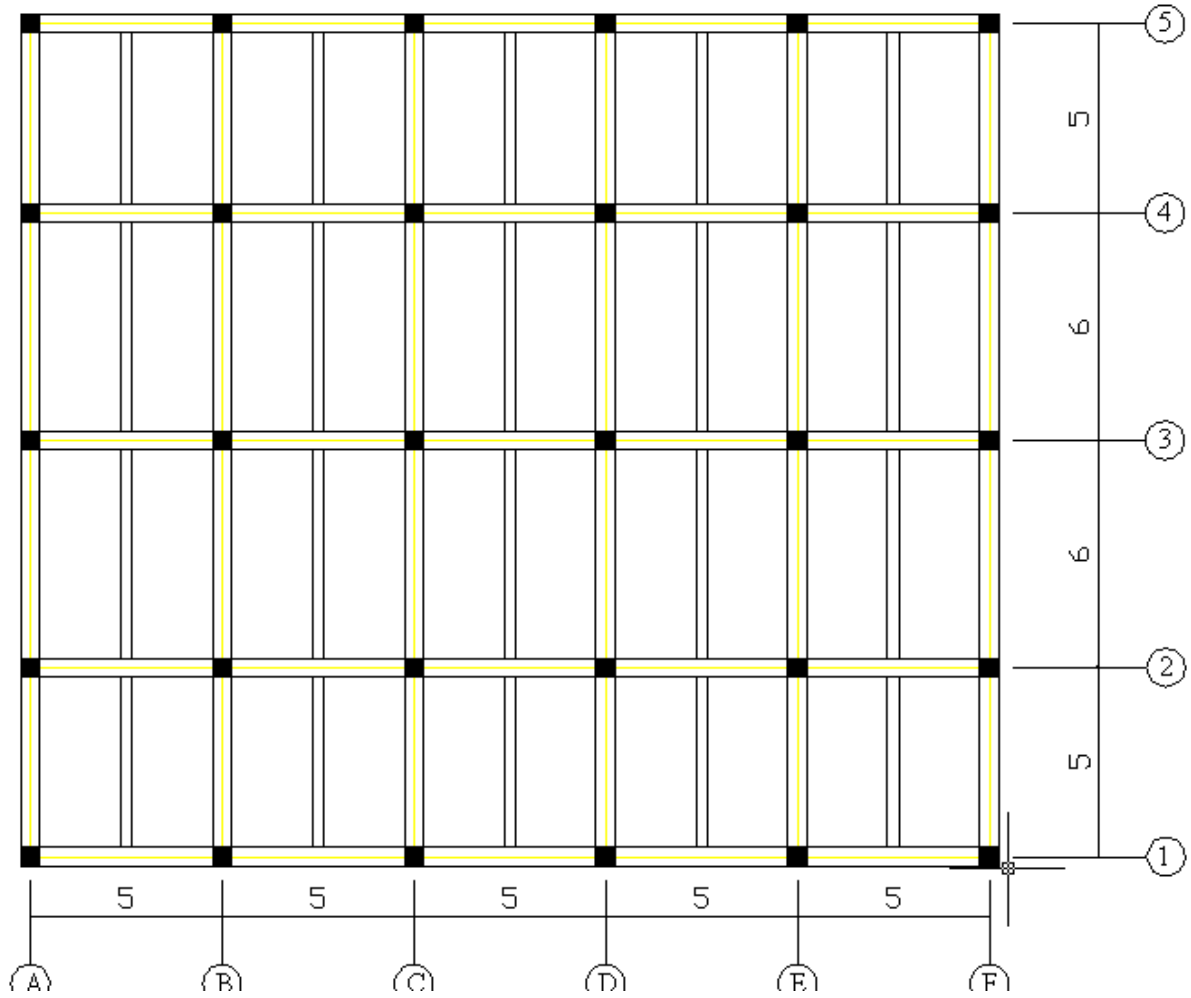


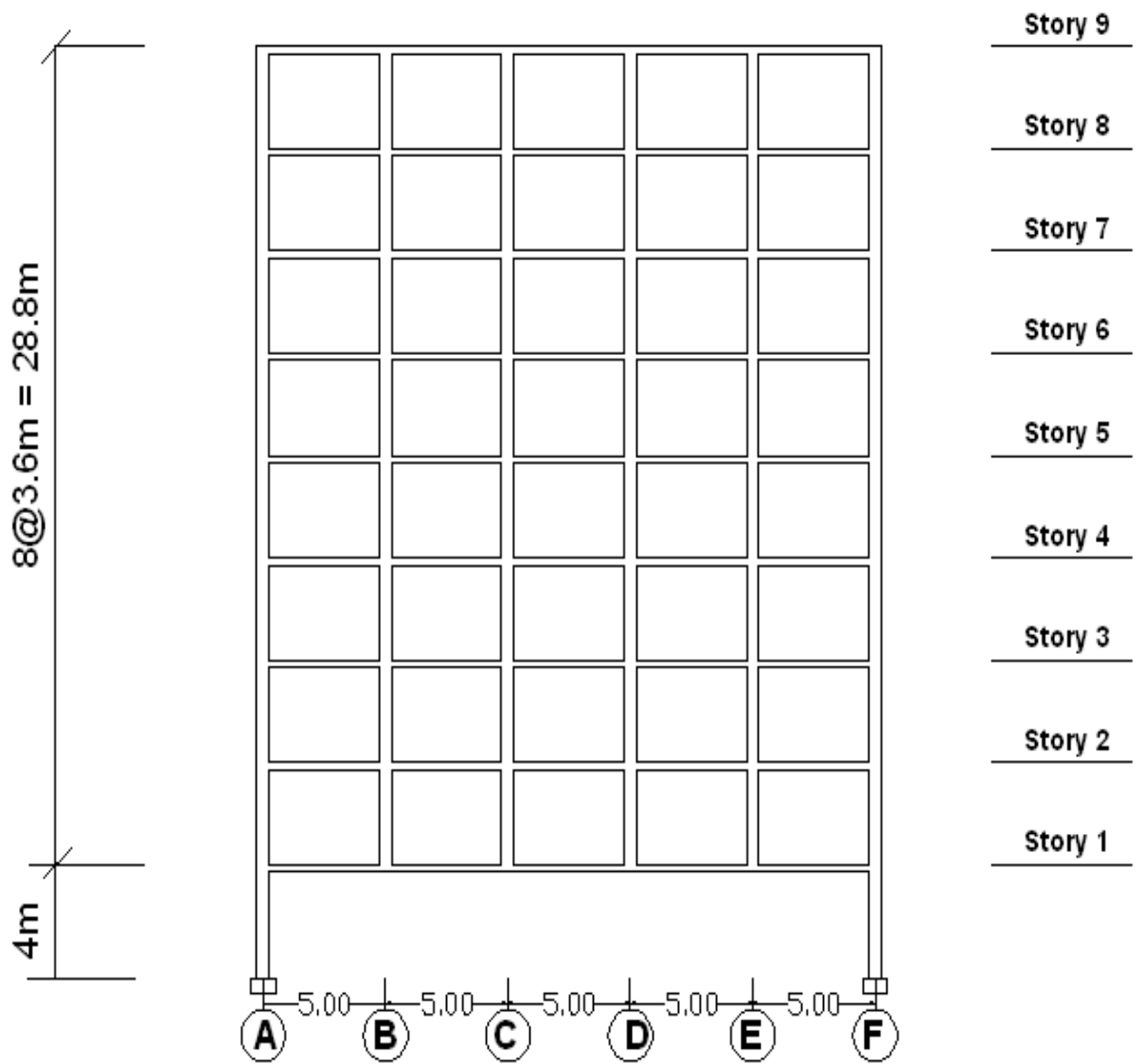
LAMPIRAN 1
GAMBAR DENAH DAN POTONGAN



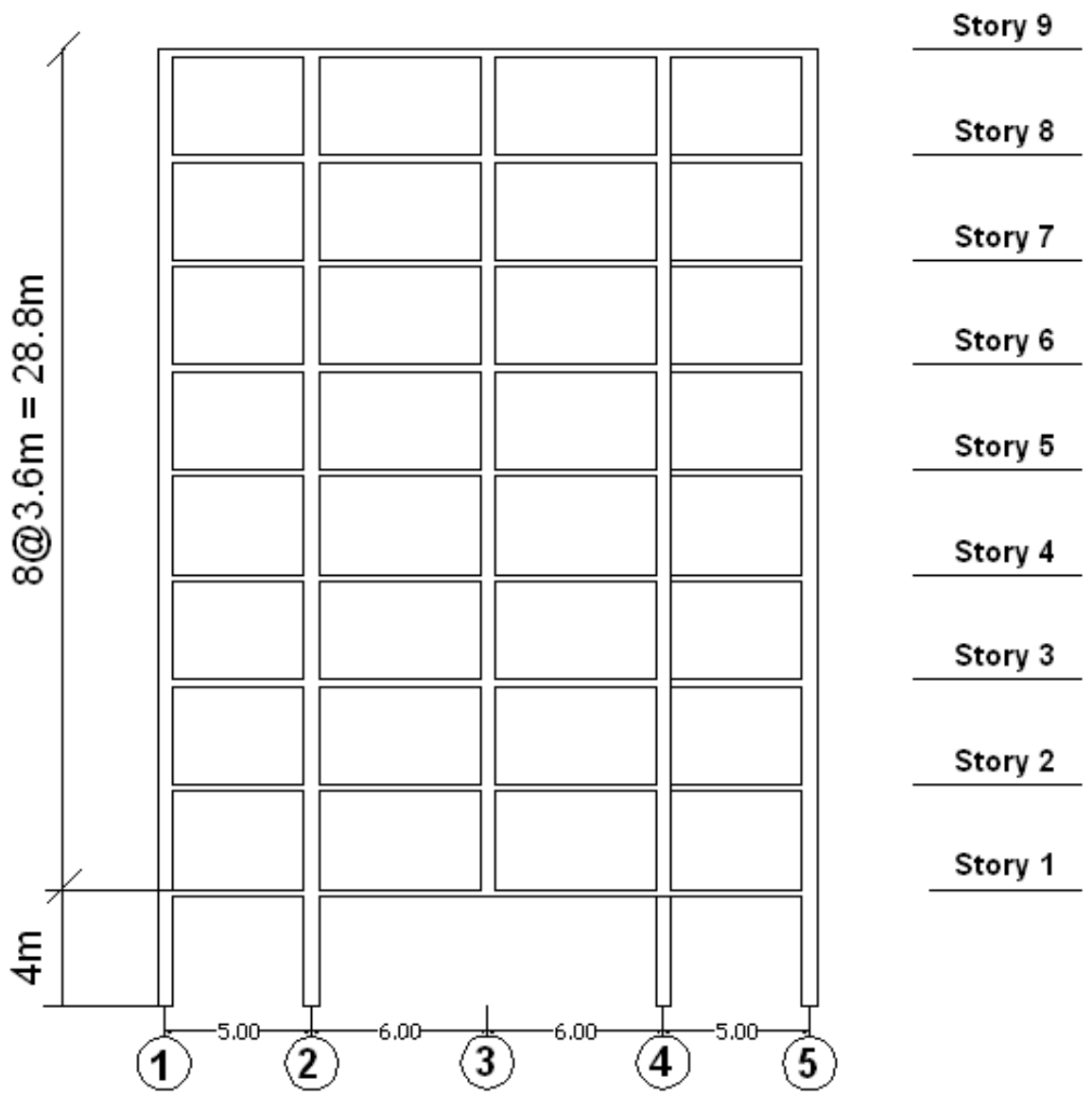
Denah Lantai 1



Denah Lantai Tipikal Lantai 2,3,4,5,6,7,8,9



Potongan memanjang As 3



Potongan Melintang As C

LAMPIRAN 2
HASIL *OUTPUT* PROGRAM ADAPT-PT

| ADAPT CORPORATION |
| STRUCTURAL CONCRETE SOFTWARE SYSTEM |
1733 Woodside Road, Suite 220, Redwood City, California 94061

| ADAPT-PT FOR POST-TENSIONED BEAM/SLAB DESIGN |
| Version 7.00 AMERICAN (ACI 318-02/IBC-03) |
| ADAPT CORPORATION - Structural Concrete Software System |
| 1733 Woodside Road, Suite 220, Redwood City, California 94061 |
| Phone: (650)306-2400, Fax: (650)364-4678 |
Email: Support@AdaptSoft.com, Web site: http://www.AdaptSoft.com

DATE AND TIME OF PROGRAM EXECUTION: Aug 1,2009 At Time: :59

PROJECT FILE: adapt pt

P R O J E C T T I T L E:

balok Transfer

prategang

1 - USER SPECIFIED G E N E R A L D E S I G N P A R A M E T E R S

=====
=====

CONCRETE:

STRENGTH at 28 days, for BEAMS/SLABS 40.00 N/mm²

for COLUMNS 40.00 N/mm²

MODULUS OF ELASTICITY for BEAMS/SLABS 29725.00 N/mm²

for COLUMNS 29725.00 N/mm²

CREEP factor for deflections for BEAMS/SLABS 2.00

CONCRETE WEIGHT NORMAL

SELF WEIGHT 2400.00 Kg/m³

TENSION STRESS limits (multiple of (f'c)^{1/2})

At Top 1.200

At Bottom 1.200

COMPRESSION STRESS limits (multiple of (f'c))

At all locations450

REINFORCEMENT:

YIELD Strength 400.00 N/mm²

Minimum Cover at TOP 45.00 mm

Minimum Cover at BOTTOM 45.00 mm

POST-TENSIONING:

SYSTEM BONDED

Ultimate strength of strand 1860.00 N/mm²

Average effective stress in strand (final) 1200.00 N/mm²

Strand area..... 98.700 mm²

Min CGS of tendon from TOP..... 120.00 mm

Min CGS of tendon from BOTTOM for INTERIOR spans.. 120.00 mm

Min CGS of tendon from BOTTOM for EXTERIOR spans.. 120.00 mm

Min average precompression85 N/mm²

```

-----
-----
Max spacing between strands (factor of slab depth) 8.00
Tendon profile type and support widths..... (see section 9)
ANALYSIS OPTIONS USED:
Structural system ..... BEAM
Moment of Inertia over support is ..... NOT INCREASED
Moments REDUCED to face of support ..... YES
Effective flange width consideration ..... YES
Effective flange width implementation method ..... ACI-318

```

2 - I N P U T G E O M E T R Y

2.1.1 PRINCIPAL SPAN DATA OF UNIFORM SPANS

```

-----
S F| | | TOP |BOTTOM/MIDDLE| |
P O| | | FLANGE | FLANGE | REF | MULTIPLIER
A R| LENGTH| WIDTH DEPTH| width thick.| width thick.| HEIGHT| left
right
N M| m | mm mm | mm mm | mm mm | mm |
-1-----3-----4-----5-----6-----7-----8-----9-----10----11-
----12----13-
1 1 25.00 600 1200 1200 .50 .50
-----

```

LEGEND:

- 1 - SPAN 3 - FORM
- C = Cantilever 1 = Rectangular section
- 2 = T or Inverted L section
- 3 = I section
- 4 = Extended T or L section
- 7 = Joist
- 8 = Waffle
- 11 - Top surface to reference line

2.2 - S U P P O R T W I D T H A N D C O L U M N D A T A

```

SUPPORT <----- LOWER COLUMN -----> <----- UPPER COLUMN ----->
WIDTH LENGTH B(DIA) D CBC* LENGTH B(DIA) D CBC*
JOINT mm m mm mm m mm mm
--1-----2-----3-----4-----5-----6-----7-----8-----
--9-----10---

```

```

1 600 4.00 600 600 (1) 3.60 600 600 (1)
2 600 4.00 600 600 (1) 3.60 600 600 (1)

```

*THE COLUMN BOUNDARY CONDITION CODES (CBC)

- Fixed at both ends ...(STANDARD) = 1
- Hinged at near end, fixed at far end = 2
- Fixed at near end, hinged at far end = 3
- Fixed at near end, roller with rotational fixity at far end .. = 4

 3 - I N P U T A P P L I E D L O A D I N G
 =====

<---CLASS---> <-----TYPE----->

D = DEAD LOAD U = UNIFORM P = PARTIAL UNIFORM
 L = LIVE LOAD C = CONCENTRATED M = APPLIED MOMENT
 Li= LINE LOAD

SW= SELF WEIGHT Computed from geometry input and treated as dead loading

Unit selfweight W = 2400.0 Kg/m³

Intensity (From ... To) (M or C ...At) Total on Trib

SPAN CLASS TYPE kN/m² (m m) (kN-m or kN...m) kN/m

-1-----2-----3-----4-----5-----6-----7-----8-----
 -----9-----

1 L C -418.19 5.00
 1 L C 166.96 10.00
 1 L C 141.62 15.00
 1 L C 116.28 20.00
 1 L M 846.17 5.00
 1 L M 570.83 10.00
 1 L M 670.95 15.00
 1 L M 926.95 20.00
 1 SW U .00 25.00 16.952

3.1 - LOADING AS APPEARS IN USER`S INPUT SCREEN PRIOR TO PROCESSING

=====

UNIFORM

(kN/m²), (CON. or PART.) (M O M E N T)

SPAN CLASS TYPE LINE(kN/m) (kN@m or m-m) (kN-m @ m)

-1-----2-----3-----4-----5-----6-----7-----
 8-----

1 L C -418.19 5.00
 1 L C 166.96 10.00
 1 L C 141.62 15.00
 1 L C 116.28 20.00
 1 L M 846.17 5.00
 1 L M 570.83 10.00
 1 L M 670.95 15.00
 1 L M 926.95 20.00

NOTE: SELFWEIGHT INCLUSION REQUIRED

4 - C A L C U L A T E D S E C T I O N P R O P E R T I E S

=====

4.1 For Uniform Spans and Cantilevers only

<----- Tributary Width -----> <----- Effective Width ----->

SPAN AREA Yb Yt b_eff I Yb Yt
mm^2 mm mm mm mm^4 mm mm

--1-----2-----3-----4-----5-----6-----7--
-----8---

1 720000.00 600.00 600.00 .00 .8640E+11 600.00 600.00

Note:

--- = Span/Cantilever is Nonuniform, see block 4.2

5 - D E A D L O A D M O M E N T S , S H E A R S & R E A C T I O N S

=====

< 5.1 S P A N M O M E N T S (kNm) > < 5.2 SPAN SHEARS (kN) >

SPAN M(l)* Midspan M(r)* SH(l) SH(r)

--1-----2-----3-----4-----5-----
-----6-----

1 -677.11 647.28 -677.03 -211.90 211.89

Note:

* = Centerline moments

JOINT < 5.3 REACTIONS (kN) > <- 5.4 COLUMN MOMENTS (kNm) ->

--1-----2-----Lower columns---Upper columns---

1 211.90 -320.73 -356.36

2 211.89 320.69 356.32

6 - L I V E L O A D M O M E N T S , S H E A R S & R E A C T I O N S

=====

<-- 6.1 L I V E L O A D SPAN MOMENTS (kNm) and SHEAR FORCES (kN) ->

<----- left* -----> <--- midspan ---> <----- right* -----> <--SHEAR FORCE-->

SPAN max min max min max min left right

-1-----2-----3-----4-----5-----6-----7-----
8-----9--

1 -21.83 -21.83 601.80 601.80 -531.71 -531.71 54.27 60.94

Note:

* = Centerline moments

<- 6.2 REACTIONS (kN) -> <----- 6.3 COLUMN MOMENTS (kNm) ----->

-->

<--- LOWER COLUMN ---> <--- UPPER COLUMN --->

JOINT max min max min max min

--1-----2-----3-----4-----5-----6-----
-----7-----

1 .00 -54.27 .00 -10.34 .00 -11.49

2 60.94 .00 251.86 .00 279.84 .00

Note: Block 6.1 through 6.3 values are maxima of all skipped loading cases

7 - M O M E N T S REDUCED TO FACE-OF-SUPPORT

=====

7.1 R E D U C E D DEAD LOAD MOMENTS (kNm)

SPAN <- left* -> <- midspan -> <- right* ->

--1-----2-----3-----4-----

1 -614.30 647.30 -614.20

Note:

* = face-of-support

7.2 R E D U C E D LIVE LOAD MOMENTS (kNm)

<----- left* -----> <----- midspan -----> <----- right* ----->

SPAN max min max min max min

-1-----2-----3-----4-----5-----6-----

---7-----

1 -38.12 -38.12 601.80 601.80 -513.40 -513.40

Note:

* = face-of-support

8 - SUM OF DEAD AND LIVE MOMENTS (kNm)

=====

Maxima of dead load and live load span moments combined for serviceability checks (1.00DL + 1.00LL)

<----- left* -----> <----- midspan -----> <----- right* ----->

SPAN max min max min max min

-1-----2-----3-----4-----5-----6-----

---7-----

1 -652.42 -652.42 1249.10 1249.10 -1127.60 -1127.60

Note:

* = face-of-support

9 - SELECTED POST-TENSIONING FORCES AND TENDON PROFILES

=====

9.1 PROFILE TYPES AND PARAMETERS

LEGEND:

For Span:

1 = reversed parabola

```

-----
-----
2 = simple parabola with straight portion over support
3 = harped tendon
For Cantilever:
1 = simple parabola
2 = partial parabola
3 = harped tendon
9.2 T E N D O N P R O F I L E
TYPE X1/L X2/L X3/L A/L
-----1-----2-----3-----4-----5-----
1 1 .100 .500 .100 .000
9.3 - SELECTED POST-TENSIONING FORCES AND TENDON DRAPE
=====
=====
Tendon editing mode selected: TENDON SELECTION
<----- SELECTED VALUES -----> <--- CALCULATED VALUES --->
FORCE <- DISTANCE OF CGS (mm) -> P/A Wbal Wbal
SPAN (kN/-) Left Center Right (N/mm^2) (kN/-) (%DL)
--1-----2-----3-----4-----5-----6-----7-
-----8--
1 1383.017 600.00 120.00 600.00 1.92 8.497 50
Approximate weight of strand ..... 221.6 Kg
9.35 - TENDON SELECTION DATA:
TYPE SEL. FORCE <----- TENDON EXTENTS -----
----->
(kN) <1>
--1-----2-----3---|-----|-----|-----|-----|-----|-----|-----|-----
|-----|-----|
B 5 123.02 <=====>
C 6 123.02 <=====>
9.5 R E Q U I R E D M I N I M U M P O S T - T E N S I O N I N G F O R C E S
(kN )
<- BASED ON STRESS CONDITIONS -> <- BASED ON MINIMUM P/A ->
SPAN LEFT* CENTER RIGHT* LEFT CENTER RIGHT
--1-----2-----3-----4-----5-----6-----
-----7-----
1 .00 340.33 78.84 612.00 612.00 612.00
Note:
* = face-of-support
9.6 S E R V I C E S T R E S S E S (N/mm^2) (tension shown
positive)
L E F T * C E N T E R R I G H T *

```

```

-----
SPAN TOP BOTTOM TOP BOTTOM TOP BOTTOM
-1-----2-----3-----4-----5-----6-----
----7-----

```

1 .67 -4.18 -8.11 4.27 3.97 -7.48

Note:

* = face-of-support

9.7 POST-TENSIONING B A L A N C E D M O M E N T S, SHEARS & REACTIONS

<-- S P A N M O M E N T S (kNm) --> <-- SPAN SHEARS (kN) -->

SPAN left* midspan right* SH(l) SH(r)

```

--1-----2-----3-----4-----5-----
---6-----

```

1 303.70 -358.20 303.70 .00 .00

Note:

* = face-of-support

<--REACTIONS (kN)--> <-- COLUMN MOMENTS (kNm) -->

-joint-----2-----Lower columns-----Upper columns-----

1 -.001 144.800 160.900

2 .001 -144.800 -160.900

10 - F A C T O R E D M O M E N T S & R E A C T I O N S

```

=====
=====

```

Calculated as (1.20D + 1.00L + 1.00 secondary moment effects)

10.1 FACTORED DESIGN MOMENTS (kNm)

<----- left* -----> <----- midspan -----> <----- right* ----->

SPAN max min max min max min

```

-1-----2-----3-----4-----5-----6-----
---7-----

```

1 -469.58 -469.58 1684.24 1684.24 -944.74 -944.74

Note:

* = face-of-support

10.2 SECONDARY MOMENTS (kNm)

SPAN <-- left* --> <- midspan -> <-- right* -->

```

-1-----2-----3-----4-----

```

1 305.70 305.70 305.70

Note:

* = face-of-support

```

-----
10.3 FACTORED REACTIONS 10.4 FACTORED COLUMN MOMENTS (kNm)
(kN) <-- LOWER column --> <-- UPPER column -->
JOINT max min max min max min
-1-----2-----3-----4-----5-----6-----
---7-----
1 254.28 200.01 -240.04 -250.38 -266.78 -278.27
2 315.22 254.28 491.94 240.04 546.46 266.66
11 - M I L D S T E E L
=====

```

```

=====
SPECIFIC CRITERIA for ONE-WAY or BEAM SYSTEM
- Minimum steel ..... 0.004A
- Moment capacity > factored (design) moment
Support cut-off length for minimum steel(length/span) ... .17
Span cut-off length for minimum steel(length/span) ... .33
Top bar extension beyond where required ..... 300.00 mm
Bottom bar extension beyond where required ..... 300.00 mm
REINFORCEMENT based on NO REDISTRIBUTION of factored moments
-----

```

```

-----
11.1 TOTAL WEIGHT OF REBAR = 123.8 Kg AVERAGE = 8.3 Kg/m^2
TOTAL AREA COVERED = 15.00 m^2
11.2.1 S T E E L A T M I D - S P A N
T O P B O T T O M
As DIFFERENT REBAR CRITERIA As DIFFERENT REBAR CRITERIA
SPAN (mm^2) <---ULT-----MIN--D+.25L-> (mm^2) <---ULT-----MIN--
D+.25L->

```

```

--1-----2-----3-----4-----5-----6-----7-----
8-----9-----
1 454 ( 454 0 0) 993 ( 993 0 0)
11.3.1 S T E E L A T S U P P O R T S
T O P B O T T O M
As DIFFERENT REBAR CRITERIA As DIFFERENT REBAR CRITERIA
JOINT (mm^2) <---ULT-----MIN--D+.25L-> (mm^2) <---ULT-----MIN--
D+.25L->

```

```

--1-----2-----3-----4-----5-----6-----7-----
8-----9-----
1 0 ( 0 0 0) 0 ( 0 0 0)
2 69 ( 69 0 0) 0 ( 0 0 0)
11.2.2 & 11.3.2 LISTING OF THE ENTIRE PROVIDED REBAR
-----

```

```

SPAN ID LOCATION NUM BAR LENGTH [mm] AREA [mm^2]
--1-----2-----3-----4-----5-----6-----7-----
1 1 T 1 # 25 x 20600 510
1 2 B 1 # 25 x 10600 510
1 3 B 1 # 25 x 600 510
-----

```

Notes:

 Bar location - T = Top, B = Bottom.

NUM - Number of bars.

Refer to tables 11.5.1,11.5.2 and PTsum graphical display for positioning of bars.

11.5.1 ARRANGEMENT OF TOP BARS

----- ----- TOP STEEL -----			
SPAN	ID	LOCATION	NUM BAR LENGTH [mm]
--1----	--2-----	3-----	---4---5-----6-----
1		1 RIGHT	1 # 25 x 20600
----- -----			

11.5.2 ARRANGEMENT OF BOTTOM BARS

----- ----- BOTTOM STEEL -----			
SPAN	ID	LOCATION	NUM BAR LENGTH [mm]
--1----	--2-----	3-----	---4---5-----6-----
1		2 CENTER	1 # 25 x 10600
1		3 CENTER	1 # 25 x 600
----- -----			

12 - S H E A R D E S I G N FOR BEAMS AND ONE-WAY SLAB SYSTEMS

=====
 =====

No shear reinforcement required

13 - MAXIMUM S P A N D E F L E C T I O N S
=====

Concrete`s modulus of elasticity $E_c = 29725 \text{ N/mm}^2$
Creep factor $K = 2.00$
Ieffective/Igross...(due to cracking)..... $K = 1.00$
Where stresses exceed $0.5(f_c')^{1/2}$ cracking of section is allowed
for.

Values in parentheses are (span/max deflection) ratios
<.....DEFLECTION ARE ALL IN mm , DOWNWARD POSITIVE.....>
SPAN DL DL+PT DL+PT+CREEP LL DL+PT+LL+CREEP
-1-----2-----3-----4-----5-----6--

1 12.9 6.2 18.6(1340) 9.5(2628) 28.2(887)

NOTE: Tensile stresses calculated exceeded the
limit for which a bilinear elastic modulus for reduction
of gross moment of inertia due to cracking is valid.
Hence, no reduction in moment of inertia is implemented

14 - I N I T I A L CONDITION STRESS CHECK & REINFORCEMENT
REQUIREMENTS

=====
14.1 Parameters specified as input for initial stress checks:
Tensile stresses divided by (f`c)^1/2
Concrete f`c (initial/final) .75 Top fiber50
PT force (initial/final) ... 1.15 Bottom fiber50
Dead loading (initial/final) 1.00
Live loading (initial/final) 1.00 Compression as ratio of f`c
.60

Note: Reinforcement reported in this data block is in
addition to that reported in data block 11 for
minimum strength reinforcement required by code.

14.2.1 SELECTION OF REBAR G R O U P 1 (REFER TO 14.2.3 FOR
POSITION)

<----- TOP STEEL -----> <----- BOTTOM STEEL ----->
SPAN (mm^2) <-- SELECTION --> (mm^2) <-- SELECTION -->
--1-----2-----3---4-----5-----6-----7---8--
----9-----

1 2739 6 #25 x 3100 mm 3672 8 #25 x10600 mm

14.2.2 SELECTION OF REBAR G R O U P 2 (REFER TO 14.2.3 FOR
POSITION)

<----- TOP STEEL -----> <----- BOTTOM STEEL ----->
JOINT (mm^2) <-- SELECTION --> (mm^2) <-- SELECTION -->
--1-----2-----3---4-----5-----6-----7---8--
----9-----

1 0 0

2 2271 5 #25 x 1850 mm 0

14.3 Compressive stresses

COMPRESSIVE stresses are within allowable limit (.60 * f`ci)
MAXIMUM stress..... = .38 * f`ci
(f`ci = initial concrete strength)

 15 - REINFORCEMENT DUE TO MOMENTS FROM LATERAL FORCES
 =====

- o Lateral moments are considered with positive and reversed directions
- o Percentage of post-tensioning considered in resisting lateral moments= 25 %
- o Factored moments calculated are the larger from the followings equations

i) $M_u = (1.20M_d + 1.60M_l + 1.00M_{sec} + 1.00M_{lat})$

ii) $M_u = (.90M_d + .00M_l + 1.00M_{sec} + 1.00M_{lat})$

Where, M_d = dead load moments;

M_l = live load moments;

M_{sec} = secondary moments; and

M_{lat} = lateral moments.

15.1 INPUTTED LATERAL MOMENTS AND THE RESULTING COMBINED MOMENTS
 kNm

<- I N P U T -> <----- CALCULATED FACTORED SPAN MOMENTS M_u -----
 ->

LATERAL MOMENTS LEFT MID-SPAN RIGHT

span left right neg-----pos neg-----pos neg-----pos
 -1-----2-----3-----4-----5-----6-----7-----
 8-----9--

1 .00 .00 -492.45 .00 .00 2045.32 -1252.78 .00

Note: Moments listed under 4,5,8,9 are reduced to face-of-support, if applicable.

For distribution of moments see file LATBM.DAT

15.2 COLUMN MOMENTS AND MOMENTS TO BE TRANSFERRED AT SUPPORT

MAX COLUMN MU

JNT neg-----pos
 -1-----2-----3-----4-----5-----6-----7-----
 8-----9--

1 -492.45 .00

2 .00 1252.78

15.3 LEGENDS AND NOTES FOR MILD STEEL

Columns 2 and the like in following block list total rebar due to lateral forces. These are not in addition to other considerations reported in preceding blocks.

For details of rebar reinforcement refer to file LATSTL.DAT

15.4 SELECTION OF REBAR A T M I D - S P A N

<----- T O P S T E E L ----> <----- B O T T O M S T E E L ---->

SPAN (mm²) Ult SELECTION (mm²) Ult SELECTION
 -1----2-----3----4-----5-----6-----7-----8-----
 9-----

1 2985(2985) 6 #25 x 15600 mm 6006(6006) 12 #25 x 18100 mm

15.5 SELECTION OF REBAR AT S U P P O R T S
<----- T O P S T E E L -----> <----- B O T T O M S T E E L ---->
JNT (mm²) Ult SELECTION (mm²) Ult SELECTION
-1-----2-----3-----4-----5-----6-----7-----8-----
-9-----
1 605(605) 2 #25 x 3100 mm
2 2564(2564) 6 #25 x 5600 mm

 16 - FRICTION, ELONGATION AND LONG TERM STRESS LOSSES
 =====

16.6 LONG TERM STRESS LOSS CALCULATIONS

16.6.1 INPUT PARAMETERS :

Type of strand LOW LAX
 Modulus of elasticity of strand
 195000.00 N/mm²
 Average weight of concrete NORMAL
 Estimate age of concrete at stressing 5 days
 Modulus of elasticity of concrete at stressing
 21647.00 N/mm²
 Modulus of elasticity of concrete at 28 days
 29725.00 N/mm²
 Estimate of average relative humidity 80.00 %
 Volume to surface ratio of member 200.00
 mm

16.6.2 CALCULATED LONG-TERM STRESS LOSS(average of all tendons) :

<----- STRESS (N/mm²) ----->
 SPAN start center right
 -1-----2-----3-----4-----
 1 58.56 49.54 58.56

16.7 FRICTION AND ELONGATION CALCULATIONS

16.7.1 INPUT PARAMETERS :

Coefficient of angular friction (meu)250
 /rad
 Coefficient of wobble friction (K)0066
 /m
 Ultimate strength of strand 1860.0
 N/mm²
 Ratio of jacking stress to strand's ultimate strength800
 Anchor set 6.000
 mm
 Cross-sectional area of strand 98.700
 mm²

16.7.2 CALCULATED STRESSES(average of all tendons) :

LENGTH <TENDON HEIGHT(mm)> Horizontal ratios <-- STRESS(N/mm²)-->
 >
 SPAN m P start center right X1/L X2/L X3/L start center right
 -1-----2-----3-----4-----5-----6-----7-----8-----9-----10-----
 11-----12--
 1 25.00 1 600. 120. 600. .10 .50 .10 1164.28 1273.85 1164.28

 Note: P= tendon profile (refer to legend of data block 9)
 Stresses at each location are the average of strands after anchor
 set,
 and after long-term losses

16.8 TENDON SELECTION AND DATA:

<----- TENDON EXTENTS -----> ELONGATION Stress ratios
 TYPE OFF FORCE CAN<----- S P A N S ----->CAN LEFT RIGHT Anch.
 Max.
 <1> (mm) (mm)
 -1-----2-----3-----4-----5-----6-----
 --7-----8---

B 5 123.02 <===> 164. 3. .66 .73

C 6 123.02 <===> 164. 3. .66 .73

Note: Force is the average value per strand (kN)

Stress ratios are at anchorage (7) and maximum along tendon (8)

ADAPT STRUCTURAL CONCRETE SOFTWARE SYSTEM DATE: Aug 1,2009 TIME:
00:49

Data ID: adapt pt Output File ID: LTLOSS.DAT

=====
=====

SUMMARY OF LONG TERM STRESS LOSS AT 1/20TH POINTS

- fpi = stress in tendon at transfer

- LT Loss = Long Term Stress Loss

SPAN = 1 LENGTH = 25.00 meter

X/L X fpi LT Loss Final Stress

m N/mm² N/mm² N/mm²

.00	.00	1222.84	58.56	1164.28
.05	1.25	1242.13	60.78	1181.35
.10	2.50	1268.09	63.98	1204.11
.15	3.75	1285.07	64.25	1220.82
.20	5.00	1300.15	61.55	1238.60
.25	6.25	1315.09	59.83	1255.26
.30	7.50	1329.87	58.02	1271.85
.35	8.75	1344.49	56.58	1287.91
.40	10.00	1351.88	54.07	1297.80
.45	11.25	1337.56	50.43	1287.13
.50	12.50	1323.39	49.54	1273.85
.55	13.75	1337.56	50.43	1287.13
.60	15.00	1351.88	54.07	1297.80
.65	16.25	1344.49	56.58	1287.91
.70	17.50	1329.87	58.02	1271.85
.75	18.75	1315.09	59.83	1255.26
.80	20.00	1300.15	61.55	1238.60
.85	21.25	1285.07	64.24	1220.82
.90	22.50	1268.09	63.98	1204.11
.95	23.75	1242.13	60.78	1181.35
1.00	25.00	1222.84	58.56	1164.28

ADAPT STRUCTURAL CONCRETE SOFTWARE SYSTEM DATE: Aug 1,2009 TIME:
00:49

Data ID: adapt pt Output File ID: WBAL.DAT

POST-TENSIONING BALANCED LOADING

=====
=====

<-----TYPE----->

1 = UNIFORM 3 = PARTIAL UNIFORM

2 = CONCENTRATED 4 = APPLIED MOMENT

(Uniform) (Con. or part.) (M o m e n t)

SPAN CLASS TYPE (kN/m) (kN@m or m-m) (kN-m @ m)

-1-----2-----3-----4-----5-----6-----7-----8-----

1 1 3 42.486 .00 2.50
1 1 3 42.486 22.50 25.00
1 1 3 -10.622 2.50 12.50
1 1 3 -10.622 12.50 22.50

ADAPT STRUCTURAL CONCRETE SOFTWARE SYSTEM DATE: Aug 1,2009 TIME:
00:49

Data ID: adapt pt Output File ID: PTCGS.DAT

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SUMMARY OF TENDON HEIGHTS AT 1/20TH POINTS. Heights in each span
are measured

from the reference point. Negative number = below reference point

CGS = Centroid of tendon

SPAN = 1 LENGTH = 25.00 meter

X/L X CGS

m mm

.00 .00 600.00
.05 1.25 576.00
.10 2.50 504.00
.15 3.75 414.00
.20 5.00 336.00
.25 6.25 270.00
.30 7.50 216.00
.35 8.75 174.00
.40 10.00 144.00
.45 11.25 126.00
.50 12.50 120.00
.55 13.75 126.00
.60 15.00 144.00
.65 16.25 174.00
.70 17.50 216.00
.75 18.75 270.00
.80 20.00 336.00
.85 21.25 414.00
.90 22.50 504.00
.95 23.75 576.00
1.00 25.00 600.00

ADAPT STRUCTURAL CONCRETE SOFTWARE SYSTEM DATE: Aug 1,2009 TIME:
00:49

Data ID: adapt pt Output File ID: MOMENTS.DAT

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SUMMARY OF BENDING SPAN MOMENTS AT 1/20TH POINTS

UNITS ARE ALL IN (kNm)

Note: for LEFT CANTILEVER (if any) X/L= 0.00 is at tip of
cantilever,

and X/L= 1.00 is at first support

SPAN = 1 LENGTH = 25.00 meter

X/L X DL LL(min) LL(max) PT SECONDARY

.00	.00	-.67711E+03	-.21834E+02	-.21834E+02	.30566E+03	.30570E+03
.05	1.25	-.42548E+03	-.89673E+02	-.89673E+02	.27247E+03	.30570E+03
.10	2.50	-.20033E+03	-.15751E+03	-.15751E+03	.17289E+03	.30570E+03
.15	3.75	-.16783E+01	-.22535E+03	-.22535E+03	.48413E+02	.30570E+03
.20	5.00	.17049E+03	-.11394E+04	-.11394E+04	-.59464E+02	.30570E+03
.25	6.25	.31617E+03	-.68446E+03	-.68446E+03	-.15074E+03	.30570E+03
.30	7.50	.43537E+03	-.22956E+03	-.22956E+03	-.22543E+03	.30570E+03
.35	8.75	.52808E+03	.22534E+03	.22534E+03	-.28352E+03	.30570E+03
.40	10.00	.59430E+03	.10940E+03	.10940E+03	-.32501E+03	.30570E+03
.45	11.25	.63403E+03	.35560E+03	.35560E+03	-.34991E+03	.30570E+03
.50	12.50	.64728E+03	.60180E+03	.60180E+03	-.35821E+03	.30570E+03
.55	13.75	.63404E+03	.84800E+03	.84800E+03	-.34991E+03	.30570E+03
.60	15.00	.59431E+03	.10942E+04	.10942E+04	-.32502E+03	.30570E+03
.65	16.25	.52810E+03	.49242E+03	.49242E+03	-.28353E+03	.30570E+03
.70	17.50	.43540E+03	.56159E+03	.56159E+03	-.22544E+03	.30570E+03
.75	18.75	.31621E+03	.63077E+03	.63077E+03	-.15076E+03	.30570E+03
.80	20.00	.17054E+03	.69994E+03	.69994E+03	-.59485E+02	.30570E+03
.85	21.25	-.16249E+01	-.30318E+03	-.30318E+03	.48389E+02	.30570E+03
.90	22.50	-.20027E+03	-.37936E+03	-.37936E+03	.17286E+03	.30570E+03
.95	23.75	-.42541E+03	-.45554E+03	-.45554E+03	.27243E+03	.30570E+03
1.00	25.00	-.67703E+03	-.53171E+03	-.53171E+03	.30562E+03	.30570E+03

ADAPT STRUCTURAL CONCRETE SOFTWARE SYSTEM DATE: Aug 1,2009 TIME:
00:49

Data ID: adapt pt Output File ID: SHEARS.DAT

=====

SUMMARY OF SHEAR FORCES ALONG SPANS AT 1/20TH POINTS

UNITS ARE ALL IN (kN)

Note: for LEFT CANTILEVER (if any) X/L= 0.00 is at tip of
cantilever,

and X/L= 1.00 is at first support

SPAN = 1 LENGTH = 25.00 meter

X/L X DL LL(pos) LL(neg) PT SECONDARY

.00	.00						
.05	1.25	-.19071E+03	.54271E+02	.00000E+00	.53109E+02	.00000E+00	
.10	2.50	-.16952E+03	.54271E+02	.00000E+00	.10622E+03	.00000E+00	
.15	3.75	-.14833E+03	.54271E+02	.00000E+00	.92940E+02	.00000E+00	
.20	5.00	-.12714E+03	.00000E+00	-.36392E+03	.79663E+02	.00000E+00	
.25	6.25	-.10595E+03	.00000E+00	-.36392E+03	.66386E+02	.00000E+00	
.30	7.50	-.84761E+02	.00000E+00	-.36392E+03	.53109E+02	.00000E+00	
.35	8.75	-.63572E+02	.00000E+00	-.36392E+03	.39832E+02	.00000E+00	
.40	10.00	-.42382E+02	.00000E+00	-.19696E+03	.26555E+02	.00000E+00	
.45	11.25	-.21193E+02	.00000E+00	-.19696E+03	.13278E+02	.00000E+00	
.50	12.50	-.30404E-02	.00000E+00	-.19696E+03	.13868E-02	.00000E+00	
.55	13.75	.21187E+02	.00000E+00	-.19696E+03	-.13276E+02	.00000E+00	
.60	15.00	.42376E+02	.00000E+00	-.55339E+02	-.26553E+02	.00000E+00	
.65	16.25	.63566E+02	.00000E+00	-.55339E+02	-.39829E+02	.00000E+00	
.70	17.50	.84755E+02	.00000E+00	-.55339E+02	-.53106E+02	.00000E+00	
.75	18.75	.10594E+03	.00000E+00	-.55339E+02	-.66383E+02	.00000E+00	
.80	20.00	.12713E+03	.60941E+02	.00000E+00	-.79660E+02	.00000E+00	
.85	21.25	.14832E+03	.60941E+02	.00000E+00	-.92937E+02	.00000E+00	
.90	22.50	.16951E+03	.60941E+02	.00000E+00	-.10621E+03	.00000E+00	
.95	23.75	.19070E+03	.60941E+02	.00000E+00	-.53106E+02	.00000E+00	
1.00	25.00						


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-----
.25 6.25 1.71 ----- -5.50
.30 7.50 ----- -1.78 ----- -2.05
.35 8.75 ----- -5.21 1.32 -----
.40 10.00 ----- -4.59 .67 -----
.45 11.25 ----- -6.38 2.50 -----
.50 12.50 ----- -8.11 4.27 -----
.55 13.75 ----- -9.80 5.92 -----
.60 15.00 ----- -11.43 7.51 -----
.65 16.25 ----- -7.06 3.18 -----
.70 17.50 ----- -7.28 3.44 -----
.75 18.75 ----- -7.42 3.64 -----
.80 20.00 ----- -7.50 3.76 -----
.85 21.25 ----- -.06 ----- -3.62
.90 22.50 1.01 ----- ----- -4.64
.95 23.75 2.44 ----- ----- -6.01
1.00 25.00
STRESSES AT FACES OF SUPPORTS
=====
=====
SPAN = 1 LENGTH = 25.00 meter (Net span from .30 to 24.70 m )
<----- L L ----->
<--- D L ---> top bottom <--- P T --->
X/L X top bottom max-T max-C max-T max-C top bottom
-----
face of support at left
.01 .30 4.27 -4.27 .26 .26 -.26 -.26 -3.86 .35
face of support at right
.99 24.70 4.27 -4.27 3.57 3.57 -3.57 -3.57 -3.86 .35
<----- COMBINED ----->
top bottom
X/L X max-T max-C max-T max-C
-----
face of support at left
.01 .30 .67 ----- ----- -4.18
face of support at right
.99 24.70 3.97 ----- ----- -7.48

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ADAPT STRUCTURAL CONCRETE SOFTWARE SYSTEM DATE: Aug 1,2009 TIME:
00:49

Data ID: adapt pt Output File ID: PTREQ.DAT

=====
SUMMARY OF POST-TENSIONING REQUIRED AT 1/20TH POINTS FOR THE
ENTIRE TRIBUTARY

UNITS ARE ALL IN (kN)

Note: for LEFT CANTILEVER (if any) X/L= 0.00 is at tip of
cantilever,

and X/L= 1.00 is at first support

SPAN = 1 LENGTH = 25.00 meter

X/L X PT

.00 .00
.05 1.25 .0000E+00
.10 2.50 .0000E+00
.15 3.75 .0000E+00
.20 5.00 .0000E+00
.25 6.25 .0000E+00
.30 7.50 .0000E+00
.35 8.75 .0000E+00
.40 10.00 .0000E+00
.45 11.25 .0000E+00
.50 12.50 .3403E+03
.55 13.75 .8639E+03
.60 15.00 .1383E+04
.65 16.25 .0000E+00
.70 17.50 .0000E+00
.75 18.75 .0000E+00
.80 20.00 .0000E+00
.85 21.25 .0000E+00
.90 22.50 .0000E+00
.95 23.75 .0000E+00
1.00 25.00

SUMMARY OF POST-TENSIONING REQUIRED AT FACES OF SUPPORTS

=====
SPAN = 1 LENGTH = 25.00 meter

X/L X PT

face of support at left

.01 .30 .0000E+00

face of support at right

.99 24.70 .7884E+02

ADAPT STRUCTURAL CONCRETE SOFTWARE SYSTEM DATE: Aug 1,2009 TIME: 00:49

Data ID: adapt pt Output File ID: REBAR.DAT

SUMMARY OF REBAR REQUIRED AT 1/20TH POINTS

Note: for LEFT CANTILEVER (if any) X/L= 0.00 is at tip of cantilever,

and X/L= 1.00 is at first support

SPAN = 1 LENGTH = 25.00 meter; CLEAR from .30 to 24.70 m

X/L X <--Factored moments (kNm)--> <--Reinforcement (mm²)-->
m MAXIMUM MINIMUM TOP BOTTOM

X/L	X	Factored moments (kNm)	Reinforcement (mm ²)
.00	.00		
.05	1.25	-294.549 -294.549	.00 .00
.10	2.50	-92.206 -92.206	.00 .00
.15	3.75	78.336 78.336	.00 .00
.20	5.00	-629.112 -629.112	454.45 .01
.25	6.25	.644 .644	.00 .00
.30	7.50	598.584 598.584	.00 .00
.35	8.75	1164.736 1164.736	.00 .00
.40	10.00	1128.260 1128.260	.00 .00
.45	11.25	1422.136 1422.136	.00 .00
.50	12.50	1684.236 1684.236	.00 .00
.55	13.75	1914.548 1914.548	.00 348.03
.60	15.00	2113.072 2113.072	.00 993.15
.65	16.25	1431.840 1431.840	.00 .00
.70	17.50	1389.770 1389.770	.00 .00
.75	18.75	1315.922 1315.922	.00 .00
.80	20.00	1210.288 1210.288	.00 .00
.85	21.25	.570 .570	.00 .00
.90	22.50	-313.984 -313.984	.00 .00
.95	23.75	-660.332 -660.332	.00 .00
1.00	25.00		

REBAR REQUIRED AT FACES OF SUPPORTS

SPAN = 1 LENGTH = 25.00 meter; CLEAR from .30 to 24.70 m

X/L X <--Factored moments (kNm)--> <--Reinforcement (mm²)-->
m MAXIMUM MINIMUM TOP BOTTOM

Face	X/L	X	Factored moments (kNm)	Reinforcement (mm ²)
face of support at left	.01	.30	-472.478 -472.478	.00 .00
face of support at right				

.99 24.70 -947.699 -947.699 69.21 .00